



ENGINEERS
SURVEYORS
PLANNERS

October 13, 2021

Nate Brusik
Lighthouse Homes
PO Box 525
Riverton, Utah 84065
Natebrusik1@gmail.com
Cc: ryan@pepg.net
ryan.dummer@pepg.net

RE: Lighthouse Heights Subdivision - Site Retaining Wall (LEI #2021-2576)

To Whom It May Concern:

Our services have been requested to design the site retaining wall to be installed along the north side of the storm water retention basin in the Lighthouse Heights Subdivision in Elk Ridge, Utah. See the attached plan, details, and calculations for our recommendations.

All site and soil design (e.g. grading, drainage, etc.) is by others and LEI Engineers & Surveyors, Inc. assumes liability for the structural design of the aforementioned site retaining wall only. Information provided in the CMT Engineering Laboratories Geotechnical Engineering Study (project #15559 dated 11/24/2020) and the PEPG Consulting Lighthouse Heights Subdivision Grading & Drainage Plan (sheet C4.0 dated 8/11/2021) were used for the design of the site retaining wall.

Please call if you have any questions or concerns. Thank you.

Sincerely,

Jordan M. West, E.I.T.
LEI Consulting Engineers & Surveyors, Inc.

Attachments

Reviewed by:

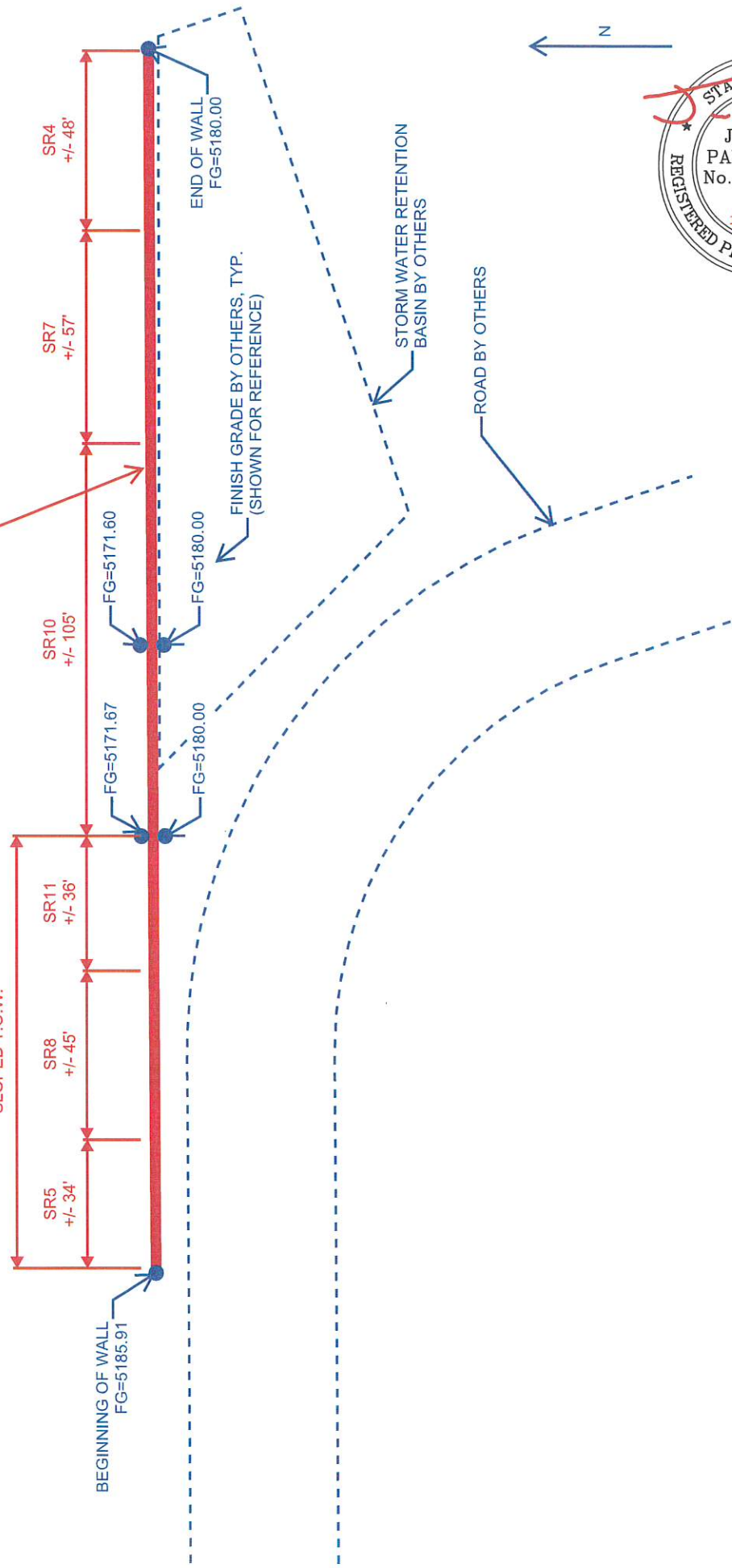
Jared R. Palfreyman,
Principal



- Civil Engineering
- Structural Engineering
- Surveying
- Land Planning
- Landscape Architecture

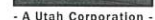
CONCRETE SITE RETAINING WALL PER CORRESPONDING DETAILS. WALL LOCATION AND FINISHED GRADE BY OTHERS. DIMENSIONS SHOWN TO FOOTING STEPS ARE APPROXIMATE; CONTRACTOR SHALL VERIFY ON SITE BASED ON FINISHED GRADE. ADDITIONAL, INTERMEDIATE FOOTING STEPS MAY OCCUR AT CONTRACTORS DISCRETION

SLOPED T.O.W.



SITE RETAINING WALL PLAN

SCALE: 1" = 40'

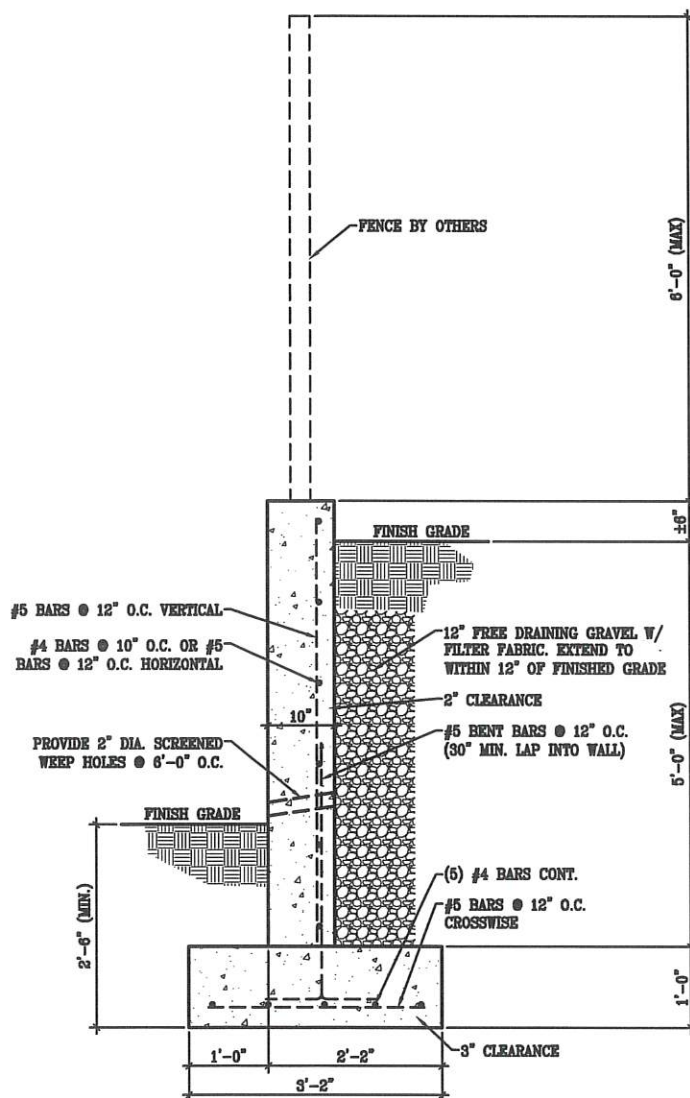


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www.lel-eng.com

NOTES:

1. f'_c (WALL) = 4,500 PSI
2. f'_c (FOOTING) = 2,500 PSI
3. F_y = 60,000 PSI
4. FOOTINGS & FOUNDATIONS, EXCAVATIONS, GRADING, & FILL SHALL COMPLY WITH THE PROVISIONS OF THE GEOTECHNICAL REPORT
5. CONTRACTOR IS RESPONSIBLE FOR ALL REQUIRED WATER STOPS, FLASHING, WATER PROOFING, MOISTURE PROTECTION, ETC.



1 5'-0" RETAINING WALL

DIMENSIONS SHOWN ON THE STRUCTURAL PLANS ARE FOR CONVENIENCE ONLY. VERIFY ALL DIMENSIONS WITH THE CURRENT ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION.

ELK RIDGE, UTAH

SITE RETAINING WALLS

DRAWN BY:

JMW

SCALE:

NTS

DATE:
10/12/2021

LEI PROJECT #:

2021-2576

SHEET

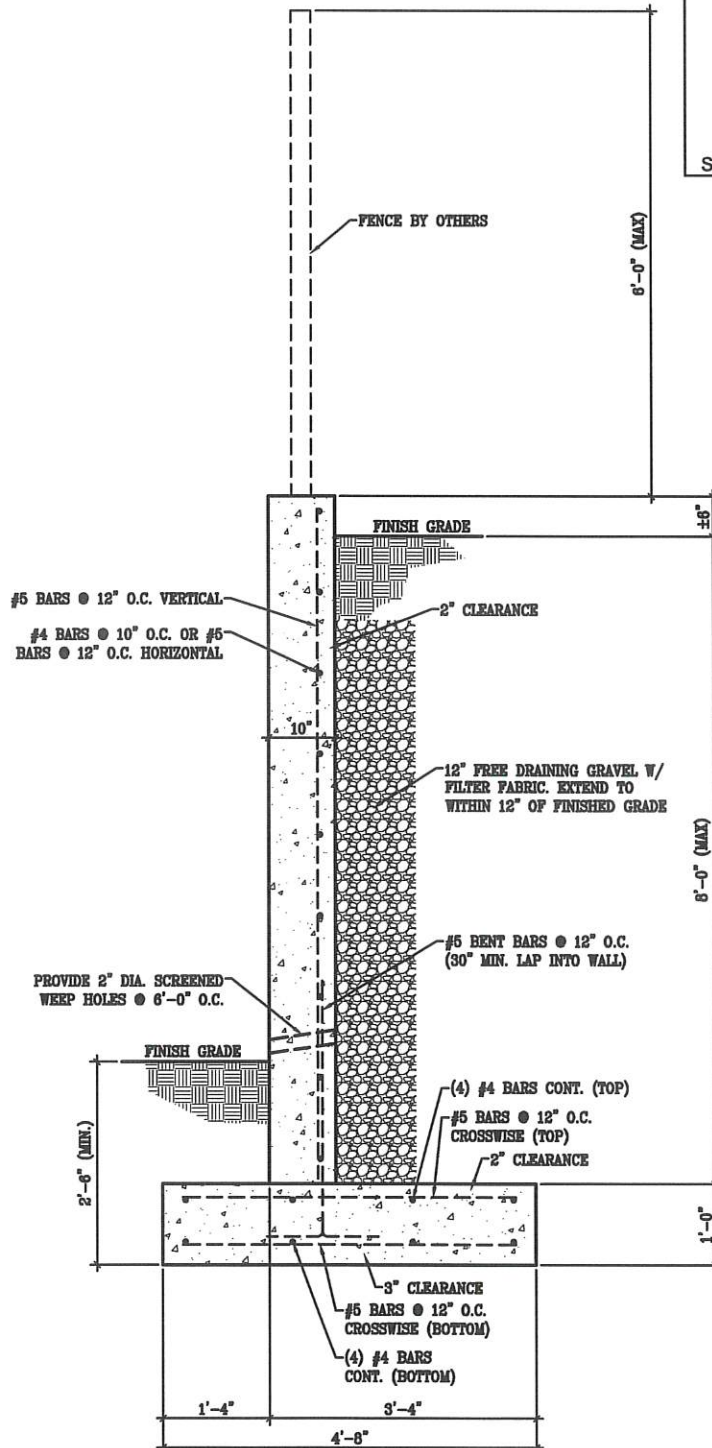
SR5



STRUCTURAL ELEMENTS ONLY

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2 8'-0" RETAINING WALL

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LIGHTHOUSE HEIGHTS SUBDIVISION

ELK RIDGE, UTAH

SITE RETAINING WALLS

DRAWN BY:

JMW

SCALE:

NTS

DATE:

10/12/2021

LEI PROJECT #:

2021-2576

SHEET

SR8



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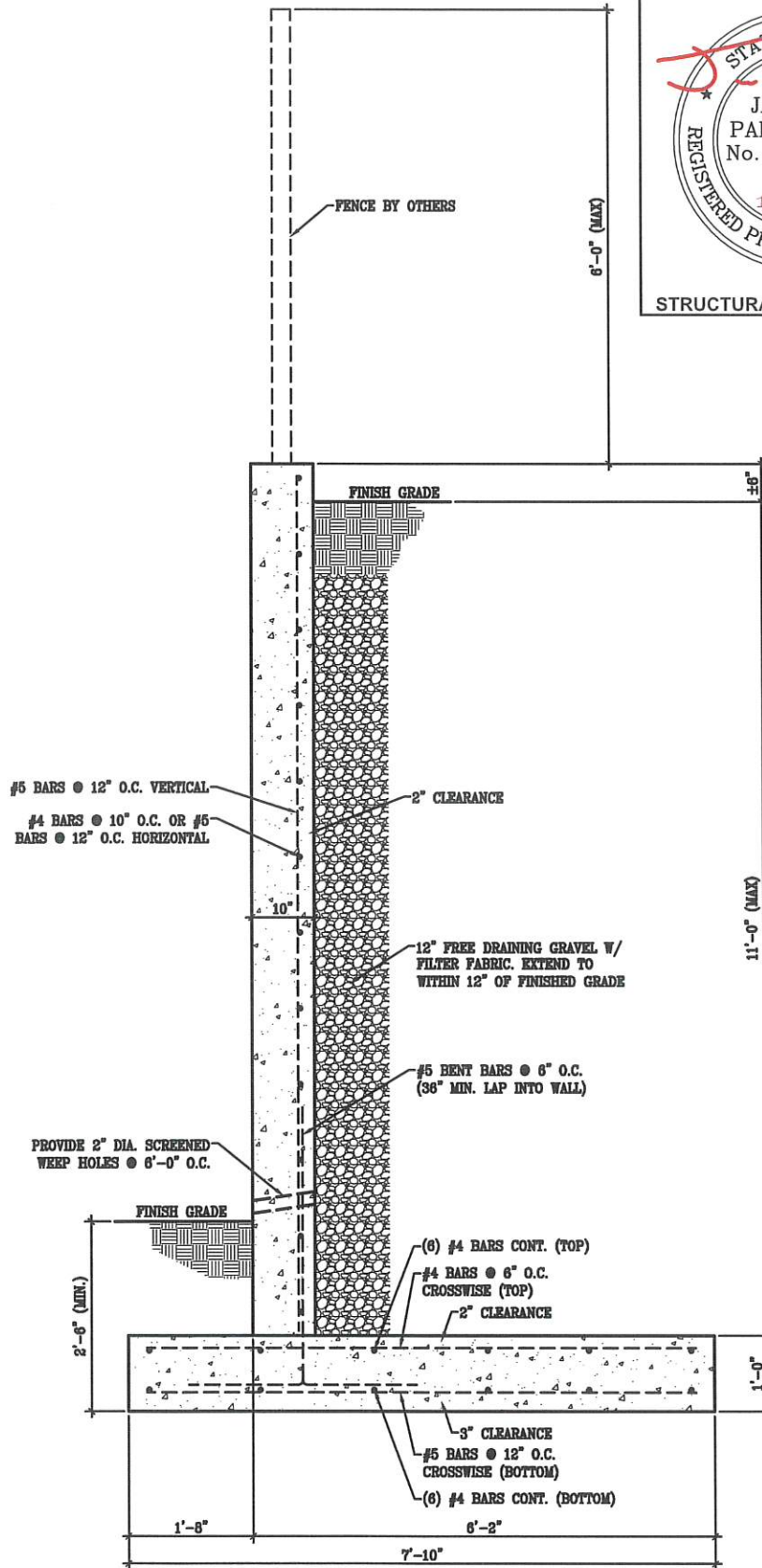
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3 11'-0" RETAINING WALL

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LIGHTHOUSE HEIGHTS SUBDIVISION

ELK RIDGE, UTAH

SITE RETAINING WALLS

DRAWN BY:

JMW

SCALE:

NTS

DATE:

10/12/2021

LEI PROJECT #:

2021-2576

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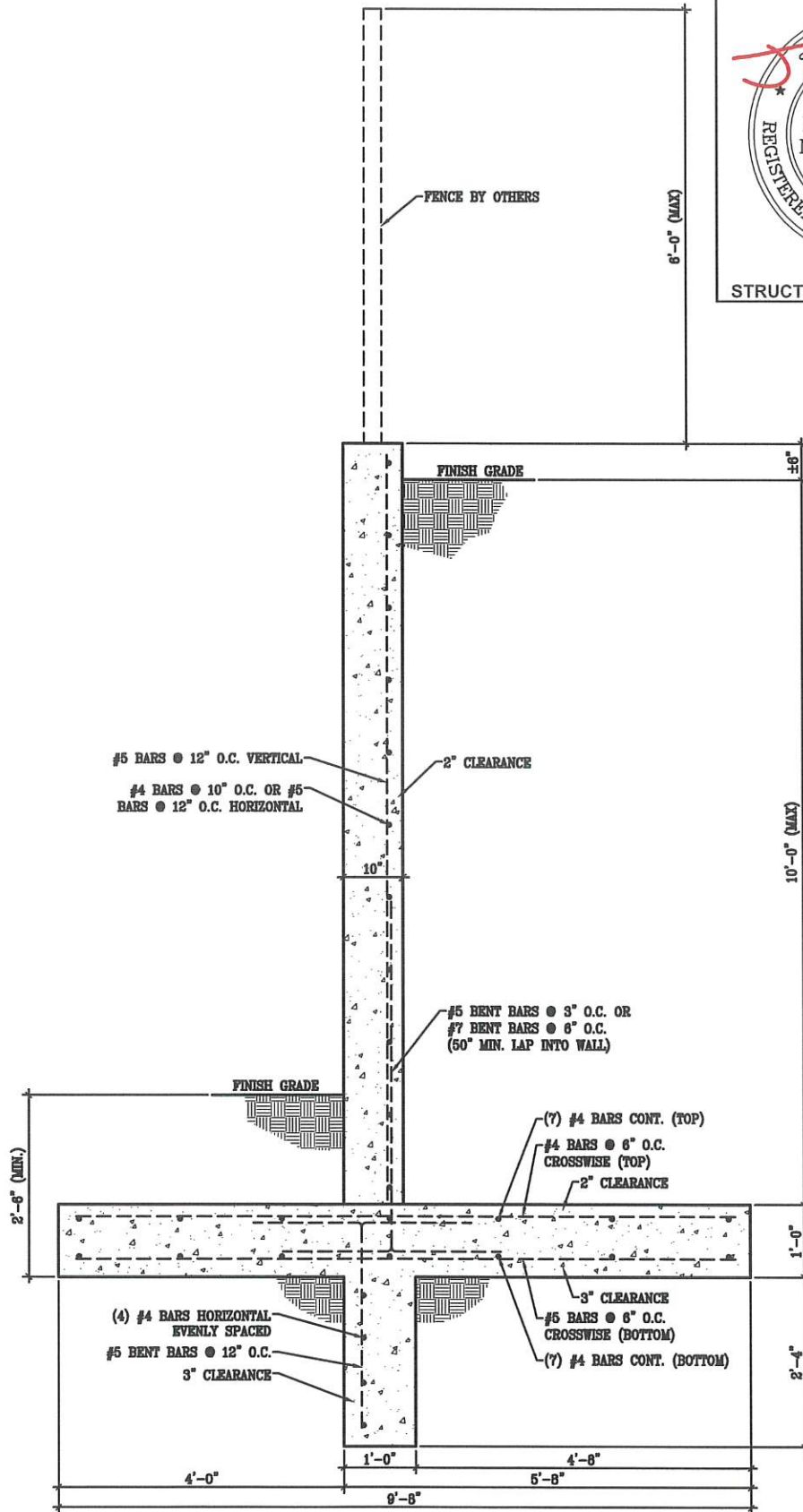
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4 10'-0" RETAINING WALL

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LIGHHOUSE HEIGHTS SUBDIVISION

ELK RIDGE, UTAH

SITE RETAINING WALLS

DRAWN BY:

JMW

SCALE:

NTS

DATE:

10/12/2021

LEI PROJECT #:

2021-2576

SHEET

SR10



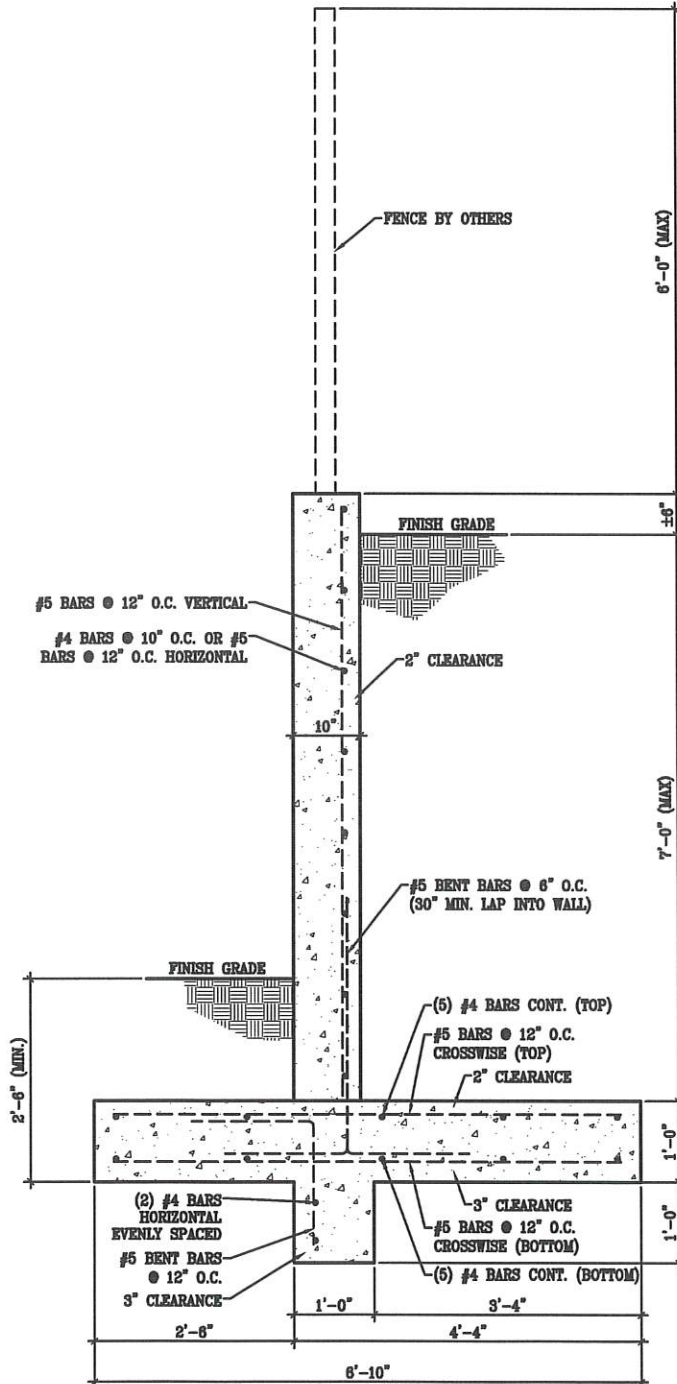
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5 7'-0" RETAINING WALL

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LIGHHOUSE HEIGHTS SUBDIVISION

ELK RIDGE, UTAH

SITE RETAINING WALLS

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SCALE:

NTS

DATE:

10/12/2021

LEI PROJECT #:

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SR7



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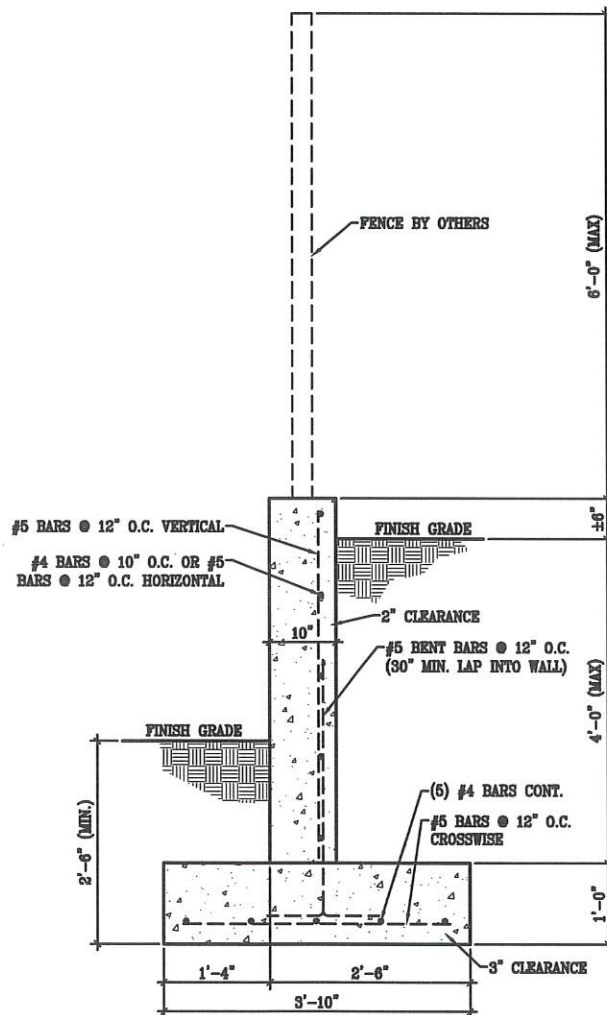
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6 4'-0" RETAINING WALL

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LIGHTHOUSE HEIGHTS SUBDIVISION

ELK RIDGE, UTAH

SITE RETAINING WALLS

DRAWN BY:

JMW

SCALE:

NTS

DATE:

10/12/2021

LEI PROJECT #:

2021-2576

SHEET

SR4



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PROJECT: _____

JOB #: _____

SUBJECT: _____

DATE: _____

DESIGNER: _____

SHEET: _____

OF: _____

WIND LOAD

$$F = q_h G C_f A_s$$

$$= (20.4 \text{ psf})(0.85)(1.3)(15 \text{ ft})$$

$$= 22.542$$

$$F = \underline{22.6 \text{ psf}}$$

$$G = 0.85$$

$$C_f = 1.30$$

$$A_s = 1 \text{ ft}^2$$



$$q_h = 0.00256 K_z K_{zt} K_d K_e V^2$$

$$= 0.00256 (0.849)(1.0)(0.85)(1.0)(105 \text{ mph})^2$$

$$= 20.367$$

$$= \underline{20.4 \text{ psf}}$$

$$\begin{aligned} K_z &= 2.01 (z/z_g)^{2/\alpha} \\ &= 2.01 (15/900)^{2/9.5} \\ &= \underline{0.849} \end{aligned}$$

$$\alpha = 9.5$$

$$z_g = 900$$

$$K_{zt} = 1.00$$

$$K_d = 0.85$$

$$K_e = 1.00$$

$$V = 105 \text{ mph}$$



Jared Palfreyman
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3302 N. Main St
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801-798-0555

Project Name/Number : 2021-2576

Title 5'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT1 FW5

Page : 1
Date: 20 SEP 2021

This Wall in File: I:\Structural\2021 Structural Jobs\2021-2576_LHH Lighthouse Heights Retaining Wal

RetainPro (c) 1987-2019, Build 11.20.03.31

License : KW-06060294

License To : LEI Engineers and Surveyors

Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

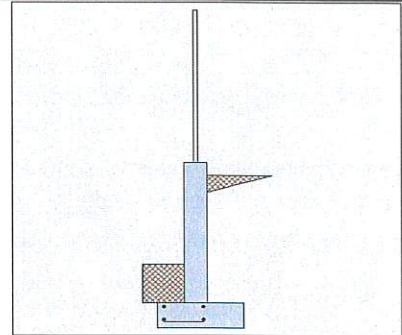
Criteria

Retained Height = 5.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 0.0 ft

Soil Data

Allow Soil Bearing = 2,000.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 425.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Service Level)
Wind on Exposed Stem = 22.6 psf
(Strength Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil at Back of Wall = 0.0 ft
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 2.32 OK
Sliding = 2.64 OK

Total Bearing Load = 1,895 lbs
...resultant ecc. = 9.61 in

Soil Pressure @ Toe = 1,615 psf OK
Soil Pressure @ Heel = 0 psf OK
Allowable = 2,000 psf

Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,262 psf
ACI Factored @ Heel = 0 psf
Footing Shear @ Toe = 4.8 psi OK
Footing Shear @ Heel = 8.3 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 718.1 lbs
less 100% Passive Force = - 1,328.1 lbs
less 100% Friction Force = - 568.6 lbs
Added Force Req'd = 0.0 lbs OK
....for 1.5 Stability = 0.0 lbs OK

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 5.50	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.217

Total Force @ Section

Service Level lbs = 81.4
Strength Level lbs = 846.9

Moment....Actual

Service Level ft-# = 244.1
Strength Level ft-# = 2,378.6
Moment.....Allowable ft-# = 10,911.3

Shear.....Actual

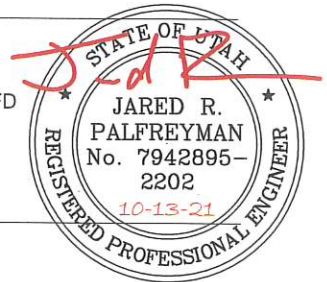
Service Level psi =
Strength Level psi = 8.6
Shear.....Allowable = 75.0
Anet (Masonry) in2 =
Rebar Depth 'd' in = 8.19

Masonry Data

f'm psi =
F_s psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0
F_y psi = 60,000.0



Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000



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Project Name/Number : 2021-2576

Title 5'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT1 FW5

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.0672 in ² /ft	
(4/3) * As :	0.0896 in ² /ft	Min Stem T&S Reinf Area 1.320 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.00 ft
Heel Width	=	2.17
Total Footing Width	=	3.17
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,262	0 psf
Mu' : Upward	= 11,642	22 ft-#
Mu' : Downward	= 2,268	746 ft-#
Mu: Design	= 781	724 ft-#
Actual 1-Way Shear	= 4.80	8.27 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: phiMn = phi'5'lambda'sqrt(f_c)'S_m
Key: No key defined

Min footing T&S reinf Area	0.82 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in





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Summary of Overturning & Resisting Forces & Moments

.....OVERTURNING.....			RESISTING.....			
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	630.0	2.00	1,260.0	Soil Over HL (ab. water tbl)	733.0	2.50	1,832.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.50	1,832.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =	88.1	9.25	815.3	Soil Over Toe =		0.50	
				Surcharge Over Toe =			
				Stem Weight(s) =	687.5	1.42	974.0
				Earth @ Stem Transitions =			
				Footing Weight =	474.9	1.58	751.8
				Key Weight =			
				Vert. Component =			
Total	= 718.1	O.T.M. =	2,075.3	Total =	1,895.4 lbs	R.M. =	4,805.8
Resisting/Overturning Ratio	= 2.32						
Vertical Loads used for Soil Pressure =	1,895.4 lbs						

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.163 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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FT1 FW5 - SEISMIC

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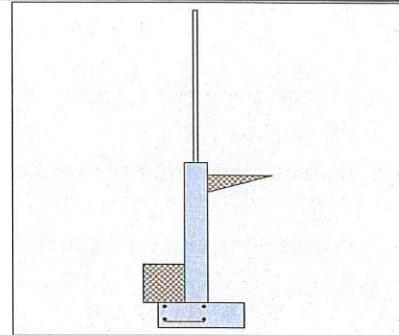
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	5.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,667.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	925.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	15.670
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Design Summary

Wall Stability Ratios		
Overturning	=	1.97 OK
Sliding	=	3.38 OK
Total Bearing Load	=	1,895 lbs
...resultant ecc.	=	11.95 in
Soil Pressure @ Toe	=	2,151 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,667 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	3,012 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	6.6 psi OK
Footing Shear @ Heel	=	9.3 psi OK
Allowable	=	75.0 psi
Sliding Calcs		
Lateral Sliding Force	=	1,024.9 lbs
less 100% Passive Force	=	- 2,890.6 lbs
less 100% Friction Force	=	- 568.6 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 5.50	Stem OK 0.00
Wall Material Above "Hi"	= Fence	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data		
fb/FB + fa/Fa	=	0.214

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,170.1

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,341.9
Moment.....Allowable	ft-# =	10,911.3

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	11.9
Shear.....Allowable	=	75.0

Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	8.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0



Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000



Jared Palfreyman
LEI Engineers and Surveyors, Inc
3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 5'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT1 FW5 - SEISMIC

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.0661 in ² /ft	
(4/3) * As :	0.0882 in ² /ft	Min Stem T&S Reinf Area 1.320 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.00 ft
Heel Width	=	2.17
Total Footing Width	=	3.17
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,012	0 psf
Mu' : Upward	= 14,652	0 ft-#
Mu' : Downward	= 2,268	746 ft-#
Mu: Design	= 1,032	746 ft-#
Actual 1-Way Shear	= 6.63	9.33 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: phiMn = phi'5'lambda'sqrt(f_c)'S_m
Key: No key defined

Min footing T&S reinf Area	0.82	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 5'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT1 FW5 - SEISMIC

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

.....OVERTURNING.....			RESISTING.....			
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	630.0	2.00	1,260.0	Soil Over HL (ab. water tbl)	733.0	2.50	1,832.2
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.50	1,832.2
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.50	
Seismic Earth Load =	394.9	3.00	1,184.7	Surcharge Over Toe =			
				Stem Weight(s) =	687.5	1.42	974.0
				Earth @ Stem Transitions =			
				Footing Weight =	474.9	1.58	751.8
				Key Weight =			
				Vert. Component =			

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.217 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe. because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 8'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT3 FW8

Page : 1
Date: 20 SEP 2021

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Cantilevered Retaining Wall

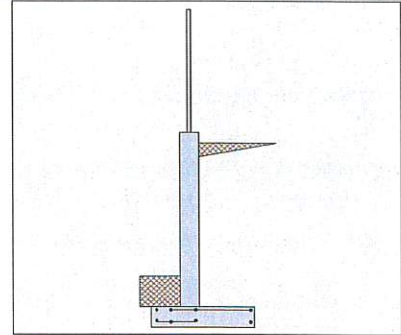
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	425.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	22.6 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Design Summary

Wall Stability Ratios

Overturning	=	3.52 OK
Sliding	=	1.67 OK

Total Bearing Load	=	3,962 lbs
...resultant ecc.	=	10.81 in

Soil Pressure @ Toe	=	1,844 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf

Soil Pressure Less Than Allowable

ACI Factored @ Toe	=	2,582 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	11.9 psi OK
Footing Shear @ Heel	=	15.1 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,505.6 lbs
less 100% Passive Force	= -	1,328.1 lbs
less 100% Friction Force	= -	1,188.6 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 8.50	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete
Design Method	=	LRFD LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.589
---------------	---	-------

Total Force @ Section

Service Level	lbs =	81.4
Strength Level	lbs =	1,938.9

Moment....Actual

Service Level	ft-# =	244.1
Strength Level	ft-# =	6,431.3
Moment.....Allowable	ft-# =	10,911.3

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	19.7
Shear.....Allowable	=	75.0
Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	8.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000





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Project Name/Number : 2021-2576
Title 8'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT3 FW8

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.1816 in ² /ft	
(4/3) * As :	0.2422 in ² /ft	Min Stem T&S Reinf Area 2.040 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2422 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.33 ft
Heel Width	=	3.33
Total Footing Width	=	4.67
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 75,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,582	0 psf
Mu' : Upward	= 24,682	968 ft-#
Mu' : Downward	= 4,030	3,861 ft-#
Mu: Design	= 1,721	2,893 ft-#
Actual 1-Way Shear	= 11.94	15.14 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 5 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide
supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Key: No key defined

Min footing T&S reinf Area	1.21	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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801-798-0555

Project Name/Number : 2021-2576

Title 8'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT3 FW8

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			Moment ft-#
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,417.5	3.00	4,252.5	Soil Over HL (ab. water tbl)	2,199.7	3.42	7,514.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.42	7,514.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =	88.1	12.25	1,079.7	Soil Over Toe =		0.67	
=				Surcharge Over Toe =			
				Stem Weight(s) =	1,062.5	1.75	1,859.0
				Earth @ Stem Transitions =			
				Footing Weighl =	699.9	2.33	1,632.9
				Key Weight =			
				Vert. Component =			
Total	= 1,505.6	O.T.M. =	5,332.2	Total =	3,962.1 lbs	R.M.=	18,776.9
Resisting/Overturning Ratio	= 3.52						
Vertical Loads used for Soil Pressure =	3,962.1 lbs						

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.159 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 8'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT3 FW8 - SEISMIC

Page : 1

Date: 20 SEP 2021

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Cantilevered Retaining Wall

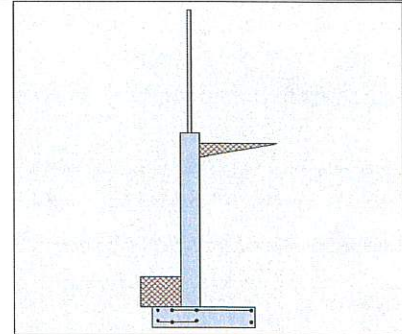
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	2,667.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	925.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	10.450
(Multiplier used on soil density)		

Uniform Seismic Force	=	94.050
Total Seismic Force	=	846.450

Design Summary

Wall Stability Ratios

Overturning	=	2.71 OK
Sliding	=	2.03 OK

Total Bearing Load	=	3,962 lbs
...resultant ecc.	=	15.62 in

Soil Pressure @ Toe	=	2,560 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,667 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	3,584 psf
ACI Factored @ Heel	=	0 psf

Footing Shear @ Toe	=	17.0 psi OK
Footing Shear @ Heel	=	22.7 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,010.0 lbs
less 100% Passive Force	=	- 2,890.6 lbs
less 100% Friction Force	=	- 1,188.6 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 8.50	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.713
---------------	---	-------

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,544.4

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	7,788.3
Moment.....Allowable	ft-# =	10,911.3

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	25.9
Shear.....Allowable	=	75.0

Anet (Masonry)	in2 =	
Rebar Depth 'd'	in =	8.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Wall Weight	psf =	125.0
Short Term Factor	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	Medium Weight
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000





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Project Name/Number : 2021-2576

Title 8'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT3 FW8 - SEISMIC

Page : 2

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.2199 in ² /ft	
(4/3) * As :	0.2933 in ² /ft	Min Stem T&S Reinf Area 2.040 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2933 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.33 ft
Heel Width	=	3.33
Total Footing Width	=	4.67
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 75,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,584	0 psf
Mu' : Upward	= 32,728	155 ft-#
Mu' : Downward	= 4,030	3,861 ft-#
Mu: Design	= 2,392	3,707 ft-#
Actual 1-Way Shear	= 17.02	22.72 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 5 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Key: No key defined

Min footing T&S reinf Area	1.21	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





Jared Palfreyman
LEI Engineers and Surveyors, Inc
3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 8'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT3 FW8 - SEISMIC

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,417.5	3.00	4,252.5	Soil Over HL (ab. water tbl)	2,199.7	3.42	7,514.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.42	7,514.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.67	
Seismic Earth Load =	592.5	4.50	2,666.3	Surcharge Over Toe =			
=				Stem Weight(s) =	1,062.5	1.75	1,859.0
Total	= 2,010.0	O.T.M. =	6,918.8	Earth @ Stem Transitions =			
				Footing Weight =	699.9	2.33	1,632.9
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio	= 2.71			Total =	3,962.1 lbs	R.M.=	18,776.9
Vertical Loads used for Soil Pressure =		3,962.1 lbs					

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.221 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT5 FW11

Page : 1
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

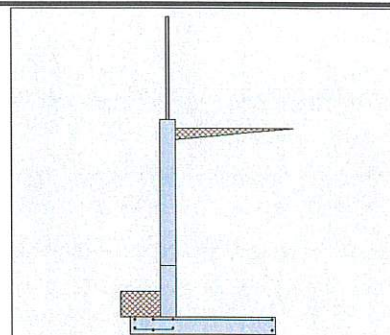
Criteria

Retained Height = 11.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 0.0 ft

Soil Data

Allow Soil Bearing = 2,000.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 425.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footings||Soil Friction = 0.300
Soil height to ignore
for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Service Level)
Wind on Exposed Stem = 22.6 psf
(Service Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil = 0.0 ft
at Back of Wall
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 3.95 OK
Sliding = 1.52 OK

Total Bearing Load = 9,065 lbs
...resultant ecc. = 9.12 in

Soil Pressure @ Toe = 1,832 psf OK
Soil Pressure @ Heel = 483 psf OK
Allowable = 2,000 psf
Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,564 psf
ACI Factored @ Heel = 677 psf
Footing Shear @ Toe = 18.8 psi OK
Footing Shear @ Heel = 14.6 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 2,666.9 lbs
less 100% Passive Force = - 1,328.1 lbs
less 100% Friction Force = - 2,719.4 lbs
Added Force Req'd = 0.0 lbs OK
....for 1.5 Stability = 0.0 lbs OK

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 11.50	Stem OK 3.00	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa = 0.589 0.697

Total Force @ Section

Service Level lbs = 135.6
Strength Level lbs = 1,938.9 3,534.9

Moment....Actual

Service Level ft-# = 406.8
Strength Level ft-# = 6,431.3 14,516.0
Moment....Allowable ft-# = 10,911.3 20,802.0

Shear....Actual

Service Level psi =
Strength Level psi = 19.7 36.0
Shear....Allowable = 75.0 75.0
Anet (Masonry) in2 =
Rebar Depth 'd' in = 8.19 8.19

Masonry Data

f'm psi =
F_s psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0 2,500.0
F_y psi = 60,000.0 60,000.0

Vertical component of active lateral soil pressure IS
NOT considered in the calculation of soil bearing

Load Factors

Building Code IBC 2018, ACI
Dead Load 1.200
Live Load 1.600
Earth, H 1.600
Wind, W 1.000
Seismic, E 1.000





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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL
Dsgnr: JMW
Description...
FT5 FW11

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.1816 in2/ft	
(4/3) * As :	0.2422 in2/ft	Min Stem T&S Reinf Area 2.040 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2422 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in2/ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.4099 in2/ft	
(4/3) * As :	0.5466 in2/ft	Min Stem T&S Reinf Area 0.720 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.4099 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.62 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in2/ft	#6@ 22.00 in #6@ 44.00 in



Footing Data

Toe Width	=	1.67 ft
Heel Width	=	6.17
Total Footing Width	=	7.83
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,564	677 psf
Mu' : Upward	= 40,473	15,711 ft-#
Mu' : Downward	= 6,295	23,205 ft-#
Mu: Design	= 2,848	7,494 ft-#
Actual 1-Way Shear	= 18.82	14.63 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Key: No key defined

Min footing T&S reinf Area	2.03	in2
Min footing T&S reinf Area per foot	0.26	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	



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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL

Dsgnr: JMW

Description...

FT5 FW11

Page : 3

Date: 20 SEP 2021

This Wall in File: I:\Structural\2021 Structural Jobs\2021-2576_LHH Lighthouse Heights Retaining Wal

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

.....OVERTURNING.....			RESISTING.....			
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,520.0	4.00	10,080.0	Soil Over HL (ab. water tbl)	6,452.5	5.17	33,331.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.17	33,331.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =	146.9	15.25	2,240.2	Soil Over Toe =		0.83	
=				Surcharge Over Toe =			
				Stem Weight(s) =	1,437.5	2.08	2,993.8
				Earth @ Stem Transitions =			
				Footing Weight =	1,174.8	3.92	4,600.5
				Key Weight =			
				Vert. Component =			
Total	= 2,666.9	O.T.M. =	12,320.2	Total =	9,064.8 lbs	R.M. =	48,696.4
Resisting/Overturning Ratio			= 3.95				
Vertical Loads used for Soil Pressure =			9,064.8 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.114 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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3302 N. Main St
Spanish Fork, UT 84660
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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL

Dsgnr: JMW

Description...

FT5 FW11 - SEISMIC

Page : 1
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

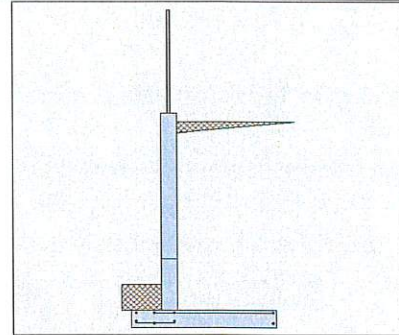
Criteria

Retained Height = 11.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 0.0 ft

Soil Data

Allow Soil Bearing = 2,667.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 927.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Earth Pressure Seismic Load

Method : Uniform
Multiplier Used = 7.840
(Multiplier used on soil density)

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Service Level)
Wind on Exposed Stem = 0.0 psf
(Service Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil = 0.0 ft
at Back of Wall
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 3.29 OK
Sliding = 1.70 OK

Total Bearing Load = 9,065 lbs
...resultant ecc. = 12.44 in

Soil Pressure @ Toe = 2,076 psf OK
Soil Pressure @ Heel = 239 psf OK
Allowable = 2,667 psf
Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,907 psf
ACI Factored @ Heel = 334 psf

Footing Shear @ Toe = 21.6 psi OK
Footing Shear @ Heel = 19.7 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 3,310.3 lbs
less 100% Passive Force = - 2,896.9 lbs
less 100% Friction Force = - 2,719.4 lbs
Added Force Req'd = 0.0 lbs OK
...for 1.5 Stability = 0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code IBC 2018, ACI
Dead Load 1.200
Live Load 1.600
Earth, H 1.600
Wind, W 1.000
Seismic, E 1.000

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 11.50	Stem OK 3.00	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa = 0.713 0.870

Total Force @ Section

Service Level lbs =
Strength Level lbs = 2,544.6 4,422.9

Moment....Actual

Service Level ft-# =
Strength Level ft-# = 7,789.2 18,114.5
Moment....Allowable ft-# = 10,911.3 20,802.0

Shear.....Actual

Service Level psi =
Strength Level psi = 25.9 45.0
Shear.....Allowable = 75.0 75.0

Anet (Masonry)

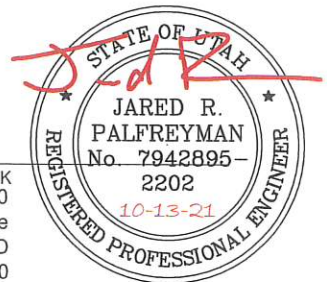
Rebar Depth 'd' in = 8.19 8.19

Masonry Data

f'm psi =
Fs psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0 2,500.0
Fy psi = 60,000.0 60,000.0





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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT5 FW11 - SEISMIC

Page : 2

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.22 in ² /ft	
(4/3) * As :	0.2933 in ² /ft	Min Stem T&S Reinf Area 2.040 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2933 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.5116 in ² /ft	
(4/3) * As :	0.6821 in ² /ft	Min Stem T&S Reinf Area 0.720 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.5116 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.62 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.67 ft
Heel Width	=	6.17
Total Footing Width	=	7.83
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm. = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,907	334 psf
Mu' : Upward	= 45,369	13,052 ft-#
Mu' : Downward	= 6,295	23,205 ft-#
Mu: Design	= 3,256	10,153 ft-#
Actual 1-Way Shear	= 21.65	19.75 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 7.32 in, #5@ 11.35 in, #6@ 16.11 in, #7@ 21.97 in, #8@ 28.93 in, #9@ 36
Key: No key defined

Min footing T&S reinf Area	2.03 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in





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Project Name/Number : 2021-2576

Title 11'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT5 FW11 - SEISMIC

Page : 3
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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

.....OVERTURNING.....			RESISTING.....			
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,520.0	4.00	10,080.0	Soil Over HL (ab. water tbl)	6,452.5	5.17	33,331.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		5.17	33,331.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.83	
Seismic Earth Load =	790.3	6.00	4,741.6	Surcharge Over Toe =			
=				Stem Weight(s) =	1,437.5	2.08	2,993.8
Total =	3,310.3	O.T.M. =	14,821.6	Earth @ Stem Transitions =			
				Footing Weight =	1,174.8	3.92	4,600.5
				Key Weight =			
				Vert. Component =			
Resisting/Overturning Ratio	= 3.29			Total =	9,064.8 lbs	R.M. =	48,696.4
Vertical Loads used for Soil Pressure =		9,064.8 lbs					

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.129 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





Jared Palfreyman
LEI Engineers and Surveyors, Inc
3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10

Page : 1
Date: 20 SEP 2021

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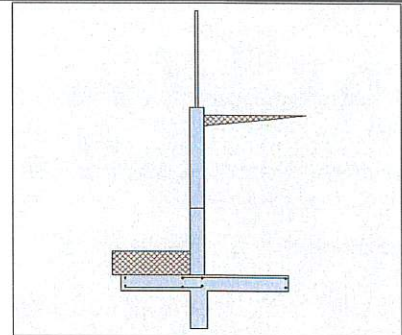
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	11.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	425.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Strength Level)
Wind on Exposed Stem	=	22.6 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Design Summary

Wall Stability Ratios

Overturning	=	2.63 OK
Sliding	=	1.51 OK

Total Bearing Load	=	9,424 lbs
...resultant ecc.	=	19.68 in

Soil Pressure @ Toe	=	1,968 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,464 psf
ACI Factored @ Heel	=	0 psf

Footing Shear @ Toe	=	51.5 psi OK
Footing Shear @ Heel	=	38.2 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	5,176.2 lbs
less 100% Passive Force	=	- 4,964.2 lbs
less 100% Friction Force	=	- 2,827.2 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 10.50	Stem OK 4.17	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	3.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa	=	0.682	0.716
---------------	---	-------	-------

Total Force @ Section

Service Level	lbs =	81.4	
Strength Level	lbs =	2,970.1	7,801.4

Moment....Actual

Service Level	ft-# =	244.1	
Strength Level	ft-# =	7,444.2	29,079.7
Moment.....Allowable	ft-# =	10,911.3	40,583.3

Shear.....Actual

Service Level	psi =		
Strength Level	psi =	30.2	79.4
Shear.....Allowable	=	75.0	94.9
Anet (Masonry)	in2 =		
Rebar Depth 'd'	in =	8.19	8.19

Masonry Data

f'm	psi =		
Fs	psi =		
Solid Grouting	=		
Modular Ratio 'n'	=		
Wall Weight	psf =	125.0	125.0
Short Term Factor	=		
Equiv. Solid Thick.	=		
Masonry Block Type	=	Medium Weight	
Masonry Design Method	=	ASD	

Concrete Data

f'c	psi =	2,500.0	4,000.0
Fy	psi =	60,000.0	60,000.0





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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL
Dsgnr: JMW
Description...
FT6 FW10

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.2102 in2/ft	
(4/3) * As :	0.2803 in2/ft	Min Stem T&S Reinf Area 1.519 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2803 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in2/ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.8212 in2/ft	
(4/3) * As :	1.095 in2/ft	Min Stem T&S Reinf Area 1.001 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.8212 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	1.24 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.7746 in2/ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	5.67
Total Footing Width	=	9.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	28.00 in
Key Distance from Toe	=	4.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,464	0 psf
Mu' : Upward	= 203,588	4,582 ft-#
Mu' : Downward	= 36,288	17,516 ft-#
Mu: Design	= 13,942	12,934 ft-#
Actual 1-Way Shear	= 51.52	38.18 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 6.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 6.33 in, #5@ 9.82 in, #6@ 13.94 in, #7@ 19.01 in, #8@ 25.03 in, #9@ 31.
Heel: #4@ 6.31 in, #5@ 9.78 in, #6@ 13.89 in, #7@ 18.94 in, #8@ 24.94 in, #9@ 31.
Key: phiMn = phi'5'lambda'sqrt(fc)'Sm

Min footing T&S reinf Area	2.51	in2
Min footing T&S reinf Area per foot	0.26	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description...

FT6 FW10

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			20,017.3	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)	1,312.9	3.67	4,813.8	Soil Over HL (bel. water tbl)	3,295.9	
Hydrostatic Force	3,775.2	3.67		Watre Table	3,015.6	7.25 23,894.0
Buoyant Force	= 603.2	4.83		Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	= 88.1	14.25	1,256.0	Soil Over Toe	=	2.00
	=			Surcharge Over Toe	=	
				Stem Weight(s)	= 1,312.5	4.42 5,796.9
				Earth @ Stem Transitions	=	
				Footing Weight	= 1,449.9	4.83 7,007.4
				Key Weight	= 350.0	4.50 1,575.0
				Vert. Component	=	
Total	= 5,779.3	O.T.M.	= 22,827.2	Total	= 9,423.9 lbs	R.M. = 60,135.2
Resisting/Overturning Ratio			= 2.63			
Vertical Loads used for Soil Pressure	=	9,423.9 lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

*Includes water table effect

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.093 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10 - SEISMIC

Page : 1
Date: 20 SEP 2021

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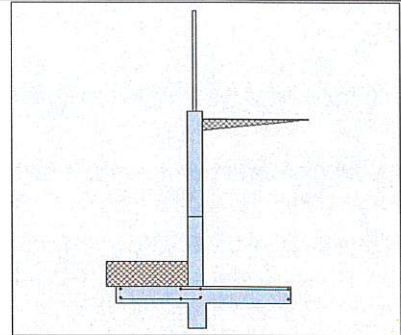
Criteria

Retained Height = 10.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 11.0 ft

Soil Data

Allow Soil Bearing = 2,667.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 925.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Earth Pressure Seismic Load

Method : Uniform
Multiplier Used = 8.550
(Multiplier used on soil density)

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Strength Level)
Wind on Exposed Stem = 0.0 psf
(Strength Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil = 0.0 ft
at Back of Wall
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 2.33 OK
Sliding = 1.75 OK

Total Bearing Load = 9,299 lbs
...resultant ecc. = 23.41 in

Soil Pressure @ Toe = 2,151 psf OK
Soil Pressure @ Heel = 0 psf OK
Allowable = 2,667 psf
Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,689 psf
ACI Factored @ Heel = 0 psf

Footing Shear @ Toe = 55.9 psi OK
Footing Shear @ Heel = 43.8 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 5,812.2 lbs
less 100% Passive Force = - 7,400.0 lbs
less 100% Friction Force = - 2,789.7 lbs
Added Force Req'd = 0.0 lbs OK
...for 1.5 Stability = 0.0 lbs OK

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 10.50	Stem OK 4.17	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	3.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa = 0.706 0.784

Total Force @ Section

Service Level lbs =
Strength Level lbs = 3,371.6 8,595.0

Moment....Actual

Service Level ft-# =
Strength Level ft-# = 7,708.7 31,835.7
Moment.....Allowable ft-# = 10,911.3 40,583.3

Shear.....Actual

Service Level psi =
Strength Level psi = 34.3 87.5
Shear.....Allowable = 75.0 94.9

Anet (Masonry) in2 =
Rebar Depth 'd' in = 8.19 8.19

Masonry Data

f'm psi =
F_s psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0 4,000.0
F_y psi = 60,000.0 60,000.0



Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code IBC 2018, ACI
Dead Load 1.200
Live Load 1.600
Earth, H 1.600
Wind, W 1.000
Seismic, E 1.000



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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description...

FT6 FW10 - SEISMIC

Page : 2

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.2177 in ² /ft	
(4/3) * As :	0.2903 in ² /ft	Min Stem T&S Reinf Area 1.519 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2903 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.8991 in ² /ft	
(4/3) * As :	1.1988 in ² /ft	Min Stem T&S Reinf Area 1.001 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.8991 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	1.24 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.7746 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	5.67
Total Footing Width	=	9.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	18.00 in
Key Distance from Toe	=	4.00 ft
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,689	0 psf
Mu' : Upward	= 218,324	2,874 ft-#
Mu' : Downward	= 36,288	17,516 ft-#
Mu: Design	= 15,170	14,642 ft-#
Actual 1-Way Shear	= 55.93	43.76 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 6.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.82 in, #5@ 9.02 in, #6@ 12.81 in, #7@ 17.47 in, #8@ 23.00 in, #9@ 29.
Heel: #4@ 6.31 in, #5@ 9.78 in, #6@ 13.89 in, #7@ 18.94 in, #8@ 24.94 in, #9@ 31.
Key: phiMn = phi'5'lambda'sqrt(f_c)'Sm

Min footing T&S reinf Area	2.51	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10 - SEISMIC

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			20,017.3	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)	1,312.9	3.67	4,813.8	Soil Over HL (bel. water tbl)	3,295.9	
Hydrostatic Force	3,775.2	3.67		Watre Table	3,015.6	7.25 23,894.0
Buoyant Force =	603.2	4.83		Sloped Soil Over Heel =		
Surcharge over Heel =				Surcharge Over Heel =		
Surcharge Over Toe =				Adjacent Footing Load =		
Adjacent Footing Load =				Axial Dead Load on Stem =		
Added Lateral Load =				* Axial Live Load on Stem =		
Load @ Stem Above Soil =				Soil Over Toe =		2.00
Seismic Earth Load =	724.2	5.50	3,983.0	Surcharge Over Toe =		
=				Stem Weight(s) =	1,312.5	4.42 5,796.9
Total	= 6,415.4	O.T.M. =	25,554.3	Earth @ Stem Transitions =		
				Footing Weight =	1,449.9	4.83 7,007.4
Resisting/Overturning Ratio		= 2.33		Key Weight =	225.0	4.50 1,012.5
Vertical Loads used for Soil Pressure =		9,298.9 lbs		Vert. Component =		
				Total =	9,298.9 lbs	R.M.= 59,572.7

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

* If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.102 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

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Description....

FT6 FW10

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

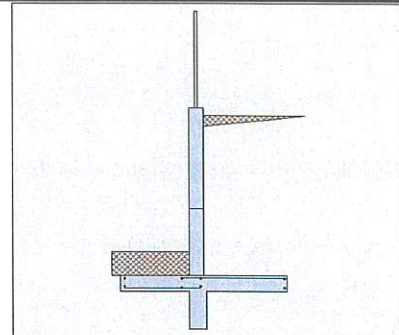
Criteria

Retained Height = 10.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 11.0 ft

Soil Data

Allow Soil Bearing = 2,000.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 425.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Strength Level)
Wind on Exposed Stem = 22.6 psf
(Strength Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil = 0.0 ft
at Back of Wall
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 2.63 OK
Sliding = 1.51 OK

Total Bearing Load = 9,424 lbs
...resultant ecc. = 19.68 in

Soil Pressure @ Toe = 1,968 psf OK
Soil Pressure @ Heel = 0 psf OK
Allowable = 2,000 psf
Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,464 psf
ACI Factored @ Heel = 0 psf

Footing Shear @ Toe = 51.5 psi OK
Footing Shear @ Heel = 38.2 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 5,176.2 lbs
less 100% Passive Force = - 4,964.2 lbs
less 100% Friction Force = - 2,827.2 lbs
Added Force Req'd = 0.0 lbs OK
...for 1.5 Stability = 0.0 lbs OK

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 10.50	Stem OK 4.17	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 7
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa = 0.682 0.806

Total Force @ Section

Service Level lbs = 81.4
Strength Level lbs = 2,970.1 7,801.4

Moment....Actual

Service Level ft-# = 244.1
Strength Level ft-# = 7,444.2 29,079.7

Moment.....Allowable

ft-# = 10,911.3 36,058.5

Shear.....Actual

Service Level psi =
Strength Level psi = 30.2 86.0

Shear.....Allowable

= 75.0 94.9

Anet (Masonry)

Rebar Depth 'd' in = 8.19 7.56

Masonry Data

f'm psi =
F_s psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f_c psi = 2,500.0 4,000.0
F_y psi = 60,000.0 60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code IBC 2018, ACI
Dead Load 1.200
Live Load 1.600
Earth, H 1.600
Wind, W 1.000
Seismic, E 1.000





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3302 N. Main St
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801-798-0555

Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT6 FW10

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.2102 in ² /ft	
(4/3) * As :	0.2803 in ² /ft	Min Stem T&S Reinf Area 1.519 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2803 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.8921 in ² /ft	
(4/3) * As :	1.1895 in ² /ft	Min Stem T&S Reinf Area 1.001 in ²
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.8921 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	1.2 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.6392 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	5.67
Total Footing Width	=	9.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	28.00 in
Key Distance from Toe	=	4.00 ft
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm. = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,464	0 psf
Mu' : Upward	= 203,588	4,582 ft-#
Mu' : Downward	= 36,288	17,516 ft-#
Mu: Design	= 13,942	12,934 ft-#
Actual 1-Way Shear	= 51.52	38.18 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 6.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 6.33 in, #5@ 9.82 in, #6@ 13.94 in, #7@ 19.01 in, #8@ 25.03 in, #9@ 31.
Heel: #4@ 6.31 in, #5@ 9.78 in, #6@ 13.89 in, #7@ 18.94 in, #8@ 24.94 in, #9@ 31.
Key: phiMn = phi*5'lambdasqrt(fc)'Sm

Min footing T&S reinf Area	2.51 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in



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Cantilevered Retaining Wall

Code: IBC 2018,ACI 318-14,TMS 402-16

Summary of Overturning & Resisting Forces & Moments

OVERTURNING.....			RESISTING.....		
Item	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			20,017.3	Soil Over HL (ab. water tbl)			
HL Act Pres (be water tbl)	1,312.9	3.67	4,813.8	Soil Over HL (bel. water tbl)	3,295.9		
Hydrostatic Force	3,775.2	3.67		Watre Table	3,015.6	7.25	23,894.0
Buoyant Force =	603.2	4.83		Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =	88.1	14.25	1,256.0	Soil Over Toe =		2.00	
=				Surcharge Over Toe =			
				Stem Weight(s) =	1,312.5	4.42	5,796.9
				Earth @ Stem Transitions =			
Total =	5,779.3	O.T.M.	= 22,827.2	Footing Weight =	1,449.9	4.83	7,007.4
				Key Weight =	350.0	4.50	1,575.0
				Vert. Component =			
Resisting/Overturning Ratio		= 2.63		Total =	9,423.9 lbs	P.M. =	60,125.2
Vertical Loads used for Soil Pressure =		9,423.9	lbs				

*Includes water table effect

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only)	0.093 in
--	----------

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10 - SEISMIC

Page : 1
Date: 20 SEP 2021

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Cantilevered Retaining Wall

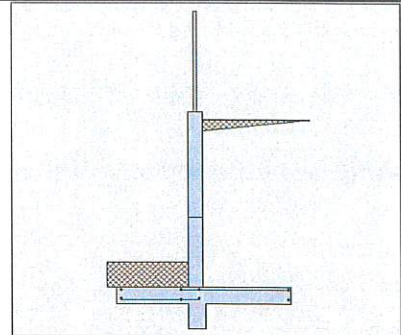
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	11.0 ft

Soil Data

Allow Soil Bearing	=	2,667.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	925.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Strength Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	8.550
(Multiplier used on soil density)		

Uniform Seismic Force	=	94.050
Total Seismic Force	=	1,034.550

Design Summary

Wall Stability Ratios

Overturning	=	2.33 OK
Sliding	=	1.75 OK

Total Bearing Load	=	9,299 lbs
...resultant ecc.	=	23.41 in

Soil Pressure @ Toe	=	2,151 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,667 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,689 psf
ACI Factored @ Heel	=	0 psf

Footing Shear @ Toe	=	55.9 psi OK
Footing Shear @ Heel	=	43.8 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	5,812.2 lbs
less 100% Passive Force	=	- 7,400.0 lbs
less 100% Friction Force	=	- 2,789.7 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 10.50	Stem OK 4.17	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 7
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa	=	0.706	0.882
---------------	---	-------	-------

Total Force @ Section

Service Level	lbs =		
Strength Level	lbs =	3,371.6	8,595.0

Moment....Actual

Service Level	ft-# =		
Strength Level	ft-# =	7,708.7	31,835.7
Moment....Allowable	ft-# =	10,911.3	36,058.5

Shear....Actual

Service Level	psi =		
Strength Level	psi =	34.3	94.7
Shear....Allowable	=	75.0	94.9

Anet (Masonry)	in2 =		
Rebar Depth 'd'	in =	8.19	7.56

Masonry Data

f'm	psi =		
Fs	psi =		
Solid Grouting	=		
Modular Ratio 'n'	=		
Wall Weight	psf =	125.0	125.0
Short Term Factor	=		
Equiv. Solid Thick.	=		
Masonry Block Type	=	Medium Weight	
Masonry Design Method	=	ASD	

Concrete Data

f'c	psi =	2,500.0	4,000.0
Fy	psi =	60,000.0	60,000.0



Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000



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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10 - SEISMIC

Page : 2

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.2177 in ² /ft	
(4/3) * As :	0.2903 in ² /ft	Min Stem T&S Reinf Area 1.519 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.2903 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.9767 in ² /ft	
(4/3) * As :	1.3022 in ² /ft	Min Stem T&S Reinf Area 1.001 in ²
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.9767 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	1.2 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.6392 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	4.00 ft
Heel Width	=	5.67
Total Footing Width	=	9.67
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	18.00 in
Key Distance from Toe	=	4.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm. = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,689	0 psf
Mu' : Upward	= 218,324	2,874 ft-#
Mu' : Downward	= 36,288	17,516 ft-#
Mu: Design	= 15,170	14,642 ft-#
Actual 1-Way Shear	= 55.93	43.76 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 6.00 in	
Heel Reinforcing	= # 4 @ 6.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 5.82 in, #5@ 9.02 in, #6@ 12.81 in, #7@ 17.47 in, #8@ 23.00 in, #9@ 29.
Heel: #4@ 6.31 in, #5@ 9.78 in, #6@ 13.89 in, #7@ 18.94 in, #8@ 24.94 in, #9@ 31.
Key: phiMn = phi'5'lambda'sqrt(f'c)'Sm

Min footing T&S reinf Area	2.51 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in





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Project Name/Number : 2021-2576

Title 10'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT6 FW10 - SEISMIC

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			20,017.3	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)	1,312.9	3.67	4,813.8	Soil Over HL (bel. water tbl)	3,295.9	
Hydrostatic Force	3,775.2	3.67		Watre Table	3,015.6	7.25 23,894.0
Buoyant Force	= 603.2	4.83		Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	=			Soil Over Toe	=	2.00
Seismic Earth Load	= 724.2	5.50	3,983.0	Surcharge Over Toe	=	
	=			Stem Weight(s)	= 1,312.5	4.42 5,796.9
				Earth @ Stem Transitions	=	
				Footing Weight	= 1,449.9	4.83 7,007.4
				Key Weight	= 225.0	4.50 1,012.5
				Vert. Component	=	
Total	= 6,415.4	O.T.M.	= 25,554.3	Total	= 9,298.9 lbs	R.M. = 59,572.7
Resisting/Overturning Ratio			= 2.33			
Vertical Loads used for Soil Pressure =			9,298.9 lbs			

*If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.102 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 7'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT4 FW7

Page : 1
Date: 20 SEP 2021

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Cantilevered Retaining Wall

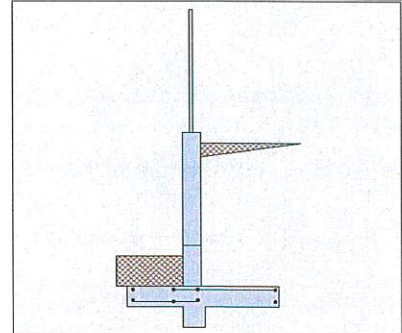
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	8.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	425.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Strength Level)
Wind on Exposed Stem	=	22.6 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Design Summary

Wall Stability Ratios

Overturning	=	2.38 OK
Sliding	=	1.51 OK

Total Bearing Load	=	5,312 lbs
...resultant ecc.	=	16.69 in

Soil Pressure @ Toe	=	1,748 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf
Soil Pressure Less Than Allowable		

ACI Factored @ Toe	=	2,215 psf
ACI Factored @ Heel	=	0 psf

Footing Shear @ Toe	=	25.4 psi OK
Footing Shear @ Heel	=	21.9 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	2,779.3 lbs
less 100% Passive Force	=	- 2,603.1 lbs
less 100% Friction Force	=	- 1,593.6 lbs
Added Force Req'd	=	0.0 lbs OK
...for 1.5 Stability	=	0.0 lbs OK

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 7.50	Stem OK 2.00	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa	=	0.482	0.555
---------------	---	-------	-------

Total Force @ Section

Service Level	lbs =	81.4	
Strength Level	lbs =	2,291.2	4,086.6

Moment....Actual

Service Level	ft-# =	244.1	
Strength Level	ft-# =	5,267.2	11,554.2
Moment.....Allowable	ft-# =	10,911.3	20,802.0

Shear.....Actual

Service Level	psi =		
Strength Level	psi =	23.3	41.6
Shear.....Allowable	=	75.0	75.0
Anet (Masonry)	in2 =		
Rebar Depth 'd'	in =	8.19	8.19

Masonry Data

f'm	psi =		
Fs	psi =		
Solid Grouting	=		
Modular Ratio 'n'	=		
Wall Weight	psf =	125.0	125.0
Short Term Factor	=		
Equiv. Solid Thick.	=		
Masonry Block Type	=	Medium Weight	
Masonry Design Method	=	ASD	

Concrete Data

f'c	psi =	2,500.0	2,500.0
Fy	psi =	60,000.0	60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000





Jared Palfreyman
LEI Engineers and Surveyors, Inc
3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 7'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT4 FW7

Page : 2

Date: 20 SEP 2021

This Wall in File: T:\Structural\2021 Structural Jobs\2021-2576_LHH Lighthouse Heights Retaining Wal

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License To : LEI Engineers and Surveyors

Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.1488 in2/ft	
(4/3) * As :	0.1983 in2/ft	Min Stem T&S Reinf Area 1.320 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in2/ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.3263 in2/ft	
(4/3) * As :	0.4351 in2/ft	Min Stem T&S Reinf Area 0.480 in2
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.3275 in2/ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.62 in2/ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in2/ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	2.50 ft
Heel Width	=	4.33
Total Footing Width	=	6.83
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	12.00 in
Key Distance from Toe	=	2.50 ft
f'c =	2,500 psi	Fy = 75,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,215	0 psf
Mu' : Upward	= 71,675	1,254 ft-#
Mu' : Downward	= 14,175	6,761 ft-#
Mu: Design	= 4,792	5,507 ft-#
Actual 1-Way Shear	= 25.37	21.86 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 5 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Key: phi Mn = phi^5 lambda sqrt(fc) Sm

Min footing T&S reinf Area	1.77	in2
Min footing T&S reinf Area per foot	0.26	in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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3302 N. Main St
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Project Name/Number : 2021-2576

Title **7'-0" RETAINING WALL**
Dsgnr: **JMW**
Description....
FT4 FW7

Page : 3
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			8,155.9	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)	694.4	2.67	1,851.7	Soil Over HL (bel. water tbl)	1,670.7	
Hydrostatic Force	1,996.8	2.67		Watre Table	1,528.7	5.08 8,492.7
Buoyant Force	= 426.4	3.42		Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	= 88.1	11.25	991.6	Soil Over Toe	=	1.25
				Surcharge Over Toe	=	
				Stem Weight(s)	= 937.5	2.92 2,734.4
				Earth @ Stem Transitions	=	
				Footing Weight	= 1,025.0	3.42 3,501.7
				Key Weight	= 150.0	3.00 450.0
				Vert. Component	=	
Total	= 3,205.7	O.T.M.	= 9,624.8	Total	= 5,311.8 lbs	R.M. = 22,949.2
Resisting/Overturning Ratio			= 2.38			
Vertical Loads used for Soil Pressure =			5,311.8 lbs			

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

*Includes water table effect

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.096 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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3302 N. Main St
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Project Name/Number : 2021-2576

Title 7'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT4 FW7 - SEISMIC

Page : 1

Date: 20 SEP 2021

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Cantilevered Retaining Wall

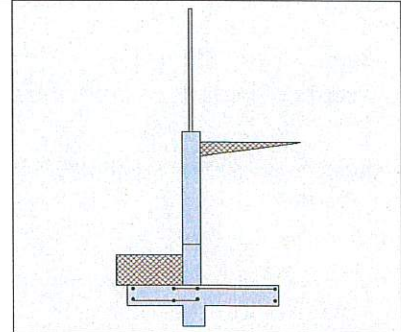
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	7.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	8.0 ft

Soil Data

Allow Soil Bearing	=	2,667.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	925.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Strength Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Earth Pressure Seismic Load

Method : Uniform		
Multiplier Used	=	11.750
(Multiplier used on soil density)		

Uniform Seismic Force	=	94.000
Total Seismic Force	=	752.000

Design Summary

Wall Stability Ratios

Overturning	=	2.14 OK
Sliding	=	2.26 OK

Total Bearing Load	=	5,312 lbs
...resultant ecc.	=	19.21 in

Soil Pressure @ Toe	=	1,950 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,667 psf
Soil Pressure Less Than Allowable		
ACI Factored @ Toe	=	2,471 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	28.5 psi OK
Footing Shear @ Heel	=	25.0 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	3,217.6 lbs
less 100% Passive Force	=	- 5,665.6 lbs
less 100% Friction Force	=	- 1,593.6 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

	3rd	2nd	Bottom
Design Height Above Ftg	ft = 7.50	Stem OK 2.00	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete	Concrete
Design Method	=	LRFD	LRFD
Thickness	=	10.00	10.00
Rebar Size	=	# 5	# 5
Rebar Spacing	=	12.00	6.00
Rebar Placed at	=	Edge	Edge

Design Data

fb/FB + fa/Fa	=	0.479	0.593
---------------	---	-------	-------

Total Force @ Section

Service Level	lbs =		
Strength Level	lbs =	2,614.3	4,597.7

Moment....Actual

Service Level	ft-# =		
Strength Level	ft-# =	5,230.3	12,351.5
Moment.....Allowable	ft-# =	10,911.3	20,802.0

Shear.....Actual

Service Level	psi =		
Strength Level	psi =	26.6	46.8
Shear.....Allowable	=	75.0	75.0

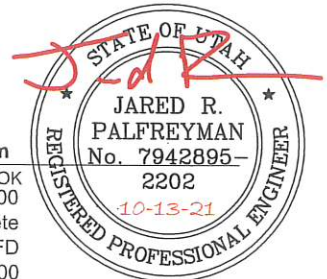
Anet (Masonry)	in2 =		
Rebar Depth 'd'	in =	8.19	8.19

Masonry Data

f'm	psi =		
Fs	psi =		
Solid Grouting	=		
Modular Ratio 'n'	=		
Wall Weight	psf =	125.0	125.0
Short Term Factor	=		
Equiv. Solid Thick.	=		
Masonry Block Type	=	Medium Weight	
Masonry Design Method	=	ASD	

Concrete Data

f'c	psi =	2,500.0	2,500.0
Fy	psi =	60,000.0	60,000.0





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801-798-0555

Project Name/Number : 2021-2576
Title 7'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT4 FW7 - SEISMIC

Page : 2
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

2nd Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.1477 in ² /ft	
(4/3) * As :	0.1969 in ² /ft	Min Stem T&S Reinf Area 1.320 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.3488 in ² /ft	
(4/3) * As :	0.4651 in ² /ft	Min Stem T&S Reinf Area 0.480 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.3488 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.62 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	2.50 ft
Heel Width	=	4.33
Total Footing Width	=	6.83
Footing Thickness	=	12.00 in
Key Width	=	12.00 in
Key Depth	=	12.00 in
Key Distance from Toe	=	2.50 ft
f _c =	2,500 psi	F _y = 75,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,471	0 psf
Mu' : Upward	= 78,486	714 ft-#
Mu' : Downward	= 14,175	6,761 ft-#
Mu: Design	= 5,359	6,046 ft-#
Actual 1-Way Shear	= 28.47	25.00 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 5 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Key: phiMn = phi'5'lambda'sqrt(f_c)'Sm

Min footing T&S reinf Area	1.77	in ²
Min footing T&S reinf Area per foot	0.26	in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:	
#4@ 9.26 in	#4@ 18.52 in	
#5@ 14.35 in	#5@ 28.70 in	
#6@ 20.37 in	#6@ 40.74 in	





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3302 N. Main St
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801-798-0555

Project Name/Number : 2021-2576

Title 7'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT4 FW7 - SEISMIC

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			8,155.9	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)	694.4	2.67	1,851.7	Soil Over HL (bel. water tbl)	1,670.7	
Hydrostatic Force	1,996.8	2.67		Watre Table	1,528.7	5.08 8,492.7
Buoyant Force	= 426.4	3.42		Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	=			Soil Over Toe	=	1.25
Seismic Earth Load	= 526.4	4.00	2,105.6	Surcharge Over Toe	=	
	=			Stem Weight(s)	= 937.5	2.92 2,734.4
				Earth @ Stem Transitions	=	
Total	= 3,644.0	O.T.M.	= 10,738.9	Footing Weight	= 1,025.0	3.42 3,501.7
				Key Weight	= 150.0	3.00 450.0
				Vert. Component	=	
Resisting/Overturning Ratio		=	2.14	Total =	5,311.8 lbs	R.M.= 22,949.2
Vertical Loads used for Soil Pressure =		5,311.8 lbs				

*If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.107 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 4'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT2 FW4

Page : 1
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Cantilevered Retaining Wall

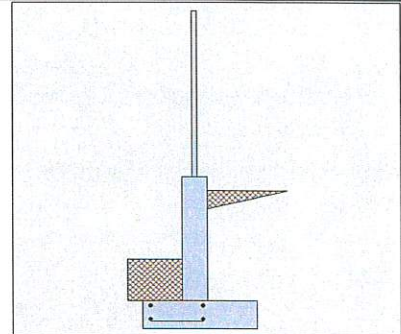
Code: IBC 2018, ACI 318-14, TMS 402-16

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	6.50 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	18.00 in
Water height over heel	=	5.0 ft

Soil Data

Allow Soil Bearing	=	2,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	425.0 psf/ft
Soil Density, Heel	=	110.00 pcf
Soil Density, Toe	=	0.00 pcf
Footings Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	0.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0 psf
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Strength Level)
Wind on Exposed Stem	=	22.6 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Line Load
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Design Summary

Wall Stability Ratios

Overturning	=	1.60 OK
Sliding	=	1.69 OK

Total Bearing Load	=	2,008 lbs
...resultant ecc.	=	14.94 in

Soil Pressure @ Toe	=	1,994 psf OK
Soil Pressure @ Heel	=	0 psf OK
Allowable	=	2,000 psf

Soil Pressure Less Than Allowable

ACI Factored @ Toe	=	2,601 psf
ACI Factored @ Heel	=	0 psf
Footing Shear @ Toe	=	10.7 psi OK
Footing Shear @ Heel	=	9.8 psi OK
Allowable	=	75.0 psi

Sliding Calcs

Lateral Sliding Force	=	1,139.4 lbs
less 100% Passive Force	=	- 1,328.1 lbs
less 100% Friction Force	=	- 602.4 lbs
Added Force Req'd	=	0.0 lbs OK
....for 1.5 Stability	=	0.0 lbs OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	IBC 2018, ACI
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 4.50	Stem OK
Wall Material Above "Ht"	= Fence	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.305
---------------	---	-------

Total Force @ Section

Service Level	lbs = 81.4
Strength Level	lbs = 1,597.9

Moment....Actual

Service Level	ft-# = 244.1
Strength Level	ft-# = 3,334.0
Moment....Allowable	ft-# = 10,911.3

Shear....Actual

Service Level	psi =
Strength Level	psi = 16.3
Shear....Allowable	= 75.0
Anet (Masonry)	in2 =
Rebar Depth 'd'	in = 8.19

Masonry Data

f'm	psi =
Fs	psi =
Solid Grouting	=
Modular Ratio 'n'	=
Wall Weight	psf = 125.0
Short Term Factor	=
Equiv. Solid Thick.	=
Masonry Block Type	= Medium Weight
Masonry Design Method	= ASD

Concrete Data

f'c	psi = 2,500.0
Fy	psi = 60,000.0





Jared Palfreyman
LEI Engineers and Surveyors, Inc
3302 N. Main St
Spanish Fork, UT 84660
801-798-0555

Project Name/Number : 2021-2576

Title 4'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT2 FW4

Page : 2

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.0942 in ² /ft	
(4/3) * As :	0.1255 in ² /ft	Min Stem T&S Reinf Area 1.080 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.33 ft
Heel Width	=	2.50
Total Footing Width	=	3.83
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,601	0 psf
Mu' : Upward	= 21,610	0 ft-#
Mu' : Downward	= 4,030	983 ft-#
Mu: Design	= 1,465	983 ft-#
Actual 1-Way Shear	= 10.74	9.83 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46

Heel: phiMn = phi'5'lambda'sqrt(f'c)'Sm

Key: No key defined

Min footing T&S reinf Area	0.99 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in





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Project Name/Number : 2021-2576
Title 4'-0" RETAINING WALL
Dsgnr: JMW
Description....
FT2 FW4

Page : 3
Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			2,260.8	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)			452.1	Soil Over HL (bel. water tbl)	454.7	
Hydrostatic Force	271.3	1.67		Watre Table	416.0	3.00 1,363.8
Buoyant Force	=	239.2	1.92	Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	=	88.1	8.25 727.2	Soil Over Toe	=	0.67
				Surcharge Over Toe	=	
				Stem Weight(s)	=	562.5 1.75 984.2
				Earth @ Stem Transitions	=	
				Footing Weight	=	575.0 1.92 1,101.9
				Key Weight	=	
				Vert. Component	=	
Total	=	1,378.6	O.T.M. = 2,937.6	Total	=	2,008.1 lbs R.M. = 4,697.8
Resisting/Overturning Ratio	= 1.60					
Vertical Loads used for Soil Pressure =	2,008.1 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

*Includes water table effect

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.152 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.





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Project Name/Number : 2021-2576

Title 4'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT2 FW4 - SEISMIC

Page : 1

Date: 20 SEP 2021

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Code: IBC 2018, ACI 318-14, TMS 402-16

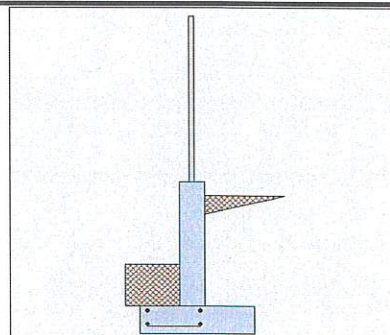
Criteria

Retained Height = 4.00 ft
Wall height above soil = 6.50 ft
Slope Behind Wall = 0.00
Height of Soil over Toe = 18.00 in
Water height over heel = 5.0 ft

Soil Data

Allow Soil Bearing = 2,667.0 psf
Equivalent Fluid Pressure Method
Active Heel Pressure = 35.0 psf/ft

Passive Pressure = 925.0 psf/ft
Soil Density, Heel = 110.00 pcf
Soil Density, Toe = 0.00 pcf
Footing||Soil Friction = 0.300
Soil height to ignore for passive pressure = 0.00 in



Surcharge Loads

Surcharge Over Heel = 0.0 psf
Used To Resist Sliding & Overturning
Surcharge Over Toe = 0.0 psf
Used for Sliding & Overturning

Axial Load Applied to Stem

Axial Dead Load = 0.0 lbs
Axial Live Load = 0.0 lbs
Axial Load Eccentricity = 0.0 in

Earth Pressure Seismic Load

Method : Uniform
Multiplier Used = 18.800
(Multiplier used on soil density)

Lateral Load Applied to Stem

Lateral Load = 0.0 #/ft
...Height to Top = 0.00 ft
...Height to Bottom = 0.00 ft
Load Type = Wind (W)
(Strength Level)
Wind on Exposed Stem = 0.0 psf
(Strength Level)

Adjacent Footing Load

Adjacent Footing Load = 0.0 lbs
Footing Width = 0.00 ft
Eccentricity = 0.00 in
Wall to Ftg CL Dist = 0.00 ft
Footing Type = Line Load
Base Above/Below Soil = 0.0 ft
at Back of Wall
Poisson's Ratio = 0.300

Design Summary

Wall Stability Ratios

Overturning = 1.55 OK
Sliding = 2.53 OK

Total Bearing Load = 2,008 lbs
...resultant ecc. = 15.51 in

Soil Pressure @ Toe = 2,146 psf OK
Soil Pressure @ Heel = 0 psf OK
Allowable = 2,667 psf
Soil Pressure Less Than Allowable

ACI Factored @ Toe = 2,799 psf
ACI Factored @ Heel = 0 psf

Footing Shear @ Toe = 11.6 psi OK
Footing Shear @ Heel = 9.8 psi OK
Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 1,380.3 lbs
less 100% Passive Force = - 2,890.6 lbs
less 100% Friction Force = - 602.4 lbs
Added Force Req'd = 0.0 lbs OK
...for 1.5 Stability = 0.0 lbs OK

Stem Construction

	2nd	Bottom
Design Height Above Ftg	ft = 4.50	Stem OK 0.00
Wall Material Above "Ht"	= Fence	Concrete
Design Method	=	LRFD
Thickness	=	10.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.276

Total Force @ Section

Service Level lbs =
Strength Level lbs = 1,827.0

Moment....Actual

Service Level ft-# =
Strength Level ft-# = 3,021.0
Moment.....Allowable ft-# = 10,911.3

Shear.....Actual

Service Level psi =
Strength Level psi = 18.6
Shear.....Allowable = 75.0
Anet (Masonry) in2 =
Rebar Depth 'd' in = 8.19

Masonry Data

f'm psi =
F_s psi =
Solid Grouting =
Modular Ratio 'n' =
Wall Weight psf = 125.0
Short Term Factor =
Equiv. Solid Thick. =
Masonry Block Type = Medium Weight
Masonry Design Method = ASD

Concrete Data

f_c psi = 2,500.0
F_y psi = 60,000.0

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code IBC 2018, ACI
Dead Load 1.200
Live Load 1.600
Earth, H 1.600
Wind, W 1.000
Seismic, E 1.000





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801-798-0555

Project Name/Number : 2021-2576

Title 4'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT2 FW4 - SEISMIC

Page : 2

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing
As (based on applied moment) :	0.0853 in ² /ft	
(4/3) * As :	0.1138 in ² /ft	Min Stem T&S Reinf Area 1.080 in ²
200bd/fy : 200(12)(8.1875)/60000 :	0.3275 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in ² /ft
0.0018bh : 0.0018(12)(10) :	0.216 in ² /ft	Horizontal Reinforcing Options :
	=====	One layer of : Two layers of :
Required Area :	0.216 in ² /ft	#4@ 10.00 in #4@ 20.00 in
Provided Area :	0.31 in ² /ft	#5@ 15.50 in #5@ 31.00 in
Maximum Area :	1.1092 in ² /ft	#6@ 22.00 in #6@ 44.00 in

Footing Data

Toe Width	=	1.33 ft
Heel Width	=	2.50
Total Footing Width	=	3.83
Footing Thickness	=	12.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f _c =	2,500 psi	F _y = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,799	0 psf
Mu' : Upward	= 22,754	0 ft-#
Mu' : Downward	= 4,030	983 ft-#
Mu: Design	= 1,560	983 ft-#
Actual 1-Way Shear	= 11.56	9.83 psi
Allow 1-Way Shear	= 75.00	40.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe: #4@ 9.25 in, #5@ 14.35 in, #6@ 20.37 in, #7@ 27.77 in, #8@ 36.57 in, #9@ 46
Heel: phiMn = phi'5'lambda'sqrt(f_c)'S_m
Key: No key defined

Min footing T&S reinf Area	0.99 in ²
Min footing T&S reinf Area per foot	0.26 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 9.26 in	#4@ 18.52 in
#5@ 14.35 in	#5@ 28.70 in
#6@ 20.37 in	#6@ 40.74 in





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3302 N. Main St
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Project Name/Number : 2021-2576

Title 4'-0" RETAINING WALL

Dsgnr: JMW

Description....

FT2 FW4 - SEISMIC

Page : 3

Date: 20 SEP 2021

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Cantilevered Retaining Wall

Code: IBC 2018, ACI 318-14, TMS 402-16

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....		
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)			2,260.8	Soil Over HL (ab. water tbl)		
HL Act Pres (be water tbl)		1.67	452.1	Soil Over HL (bel. water tbl)	454.7	
Hydrostatic Force	780.0	1.67		Watre Table	416.0	3.00 1,363.8
Buoyant Force	= 239.2	1.92		Sloped Soil Over Heel	=	
Surcharge over Heel	=			Surcharge Over Heel	=	
Surcharge Over Toe	=			Adjacent Footing Load	=	
Adjacent Footing Load	=			Axial Dead Load on Stem	=	
Added Lateral Load	=			* Axial Live Load on Stem	=	
Load @ Stem Above Soil	=			Soil Over Toe	=	0.67
Seismic Earth Load	= 329.0	2.50	822.5	Surcharge Over Toe	=	
	=			Stem Weight(s)	= 562.5	1.75 984.2
				Earth @ Stem Transitions	=	
				Footing Weight	= 575.0	1.92 1,101.9
				Key Weight	=	
				Vert. Component	=	
Total	= 1,619.4	O.T.M.	= 3,033.0	Total	= 2,008.1 lbs	R.M. = 4,697.8
Resisting/Overturning Ratio		=	1.55			
Vertical Loads used for Soil Pressure =		2,008.1 lbs				

* If seismic is included, the O.T.M. and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.163 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

